Canadian Building Envelope Science and Technology

38 Regan Road, Unit 4, Brampton, Ontario, Canada, L7A 1C6

Tel: (905) 840-2014 • Fax: (905) 840-2847 • e-mail: lab@can-best.com



LABORATORY TEST REPORT

Balcony Railing
"3091 Railing Type #6"
Aluminum and Tempered Glass

Performed in accordance with CSA A500-16 "Building Guards" ASTM E935-13 & ASTM E2353-16

Report No. L20-540-5997a

Report Date: January 4, 2021

Prepared for:

Art Aluminum Railing Technologies Ltd.

66 Rivalda Rd. Toronto, ON M9M 2M3 Canada

Respectfully submitted by:	CANADIAN BUILDING ENVELOPE	
	Science and Technology (CAN-BEST)	

Tests Supervised by:

Tariq In'airat E.I.T.

Report Authorized by:

Elie Alkhoury, M.Eng. (Building Science), P.Eng. Director, Research and Testing Services

Tay Towns

Project Manager

- This report does not constitute certification of the test product. The reported test results refer only to the specimen tested. No representation is made that other samples of similar design will feature like performance.
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1. INTRODUCTION

Canadian Building Envelope Science and Technology (CAN-BEST) was retained by Falbo Aluminum Products to carry out load testing on their glass balcony railing system. Testing was performed in accordance with Article 4.1.5.14 "Loads on Guards" of both Ontario Building Code (OBC) 2012 and National Building Code of Canada (NBCC) 2015 requirements. In addition, testing for impact load and post-breakage retention was performed in accordance with CSA A500-16 "Building Guards".

2. **DISCLAIMERS**

This report covers certain tests carried out on one guard rail specimen having specific properties, configuration and dimensions. Product performance is affected by variations in dimensions, assembly details and installation method. Consequently, the reader is advised to ensure product suitability for the intended application and conformity with all the details of the test sample described in the following section.

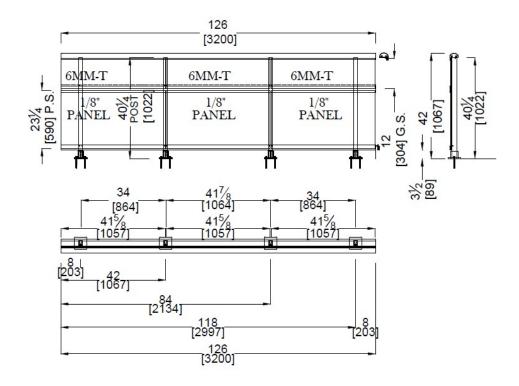
This report does not cover the rail's anticipated performance under service environmental conditions, nor the anchoring strength and stability of the substrate. No conclusions regarding concrete anchor performance or glass performance may be drawn from the reported results.

3. RAILING DESCRIPTION

"3091 Railing Type #6, Aluminum and Tempered Glass" **Designation:**

Type: Top mounted, four-post balcony railing system, having three main lower

aluminum panels and three upper tempered glass panels.





Railing Description (Cont'd.)

Panels: Aluminum - 3 mm thick (1/8") aluminum panels, 1067 mm (42") wide by 584 mm

(23") high panels with 203 mm (8") overhang.

Glass - 6 mm (0.24") tempered glass, 1067 mm (42") wide by 304 mm (12") high

panels with 203 mm (8") overhang.

Panel Support: The glass and aluminum panels were continuously supported at their

respective top and bottom rails using extruded aluminum channels lined with continuous rubber spline. An intermediate extruded aluminum H-Channel provided a continuous support to the bottom glass edge and the top

aluminum edge simultaneously.

Railing: 1070 mm (42 1/8") high top rail, 3200 mm (126") long, comprised the following:

• **Posts** - Four 914 mm (36") long extruded aluminum posts of rectangular tubular section, 50.8 mm x 38.1 mm (2.0" x 1.5") x 4.8 mm (0.19") thick; the inner posts spaced at 1067 mm (42"), and the outer posts spaced at 863 mm (34").

- *Top Rail Plate* One continuous, extruded aluminum top rail plate fastened to the top end of each post by four #10 x 1 1/2" long pan head coated TEK screws.
- *Top Rail Cap* One continuous, extruded aluminum top rail cap of irregular section, snapped onto the top rail, and clamped with a total of nine aluminum plates, 25 mm wide x 73 mm long x 0.125" thick (1" x 2 7/8" x 1/8"), positioned two at the ends, two at each side panel and three at center panel. Each plate was fastened from underneath with one 1/4-20 x 1 1/2" long pan head stainless steel machine screw.
- *Intermediate H-Channel* One continuous, extruded aluminum bypass H-Channel fastened to each post by two #10 x 3/4" pan head coated TEK screws and to the each top corner of the aluminum panels by one #10 x 3/4" pan head coated TEK screw.
- **Bottom Rail** One continuous, extruded aluminum bypass bottom rail, fastened to the bottom end of each post by two #10 x 3/4" pan head coated TEK screws.

Anchoring: Each post was fastened into an aluminum shoe using two 3/8" x 1/2" locking

screws. The shoe comprised a 3" high x 0.375" thick aluminum sleeve welded onto 125 mm x 125 mm x 10 mm (5" x 5" x 0.39") aluminum base plate. Each shoe was top anchored to the concrete slab using two 1/2" x 4 1/2" long stainless steel threaded expansion anchors to a minimum embedment depth of 3 1/4" in pre-

drilled holes.

Sampling: Railing assembly was selected and installed by the client.

Modifications: No modifications were performed on the specimen during testing in order to

attain the reported results.

Drawings: Detailed drawings, provided by the client, verified by CAN-BEST for general

conformity, are enclosed with this report (4 pages).

Building Envelope Performance



4. TEST LOADS

Report Date: January 4, 2021

Static test loads were applied in accordance with the requirements of Article 4.1.5.14. of both OBC and NBCC including a **Load Factor of 1.50**.

In addition, testing for impact resistance and post-breakage retention was performed in accordance with CSA A500 Standard. Test load schedule and a summary of CSA A500 Standard requirements are provided in Table (1) and Appendix (A) respectively.

Table (1): Test Load Schedule

OBC-2012/NBC-2015 Article 4.1.5.14 Requirements

Service Load: 1.00 kN (225lb)

Load Factor: 1.50

4.1.5.14 (1) Horizontal Load at Top of Guard

Horizontal point load applied at top rail in the outward direction shall be the greater of:

4.1.5.14 (1) (b) a concentrated load of Basic Load of 1.0 kN (225 lb) applied at any point.

<u>OR</u>

Client specified load factor applied to OBC's Basic Load:

Ultimate Load: 1.50 kN (337 lb)

\underline{OR}

4.1.5.14 (1) (c)

Basic Load: 0.75 kN/m (51 lb/ft)
Ultimate Load: 1.13 kN/m (77 lb/ft)
Post Spacing: 1067 mm (42.0")
Equivalent Load: 1.20 kN (270 lb)

4.1.5.14 (2) Horizontal Load at Infill Elements

Individual elements within the guard including solid panels and pickets, shall be designed for a load of 0.5 kN (112.5 lb) applied over an area of 100 mm x 100 mm located at any point in the element or elements as to produce the most critical effect.

Service Load: 0.5 kN (113 lb)
Ultimate Load: 0.75 kN (169 lb)

4.1.5.14 (4) Vertical Load at Top of Guard

Basic Load: 1.5 kN/m (103 lb/ft)
Ultimate Load: 2.25 kN/m (154 lb/ft)
Post Spacing: 1067 mm (42.0")
Equivalent Load: 2.40 kN (540 lb)

CSA A500-16 Requirements

5.3.6 Impact Resistance

Impact Load: 542 N.m, equivalent to 45.4 kg mass impacting at 1220 mm free fall

One hit at center of each infill panel



5.5.3 Post-Breakage Retention

Post-Breakage Load: 25% of the service wind load or 225 N, whichever is greater.

5. TEST RESULTS

Test results for static loading are provided in Table (2). Impact loading and post-breakage retention results are provided in Table (3).

Table (2): Loading Test Results (Article 4.1.5.14. of the NBC)

Test Date: October 22-28, 2020

TEST REQUIRE	MENT	Load Location	RESULTS	RATING
Service Load Top of guard at m Service Load:	ost critical location 1.00 kN (225lb)	End Post	Post Deflection (mm) <u>Under Load Permanent</u> 9.80 0.66	PASS
	1.00 kN (225lb) nent deflection: 5 mm h repeated application of	Center Post	Post Deflection (mm) <u>Under Load Permanent</u> 9.72 0.62	PASS
Ultimate Load Top of guard at me Service Load: Load Factor:	ost critical location 1.00 kN (225lb) 2.00	End Post	Post Deflection (mm) <u>Under Load Permanent</u> 22.94 0.99	PASS
Test Load: No criteria provid deflection under ledeflection after re	oad or for permanent	Center Post	Post Deflection (mm) <u>Under Load Permanent</u> 23.53 1.02	PASS
Elements Within Loads applied at a Service Load: Load Factor: Test Load:	the Guard most critical location 0.50 kN (113 lb) 2.00 1.00 kN (225b)	Infill panels at most critical location	Observations: • No glass breakage • No permanent deformation of supporting elements	PASS
	1.50 kN/m (100 lb/ft) 1.50 2.25 kN/m (154 lb/ft) 1067 mm (42.0") 2.40 kN (540 lb) in provided for deflection after	Mid-span of top rail	Top Rail deflection (mm) <u>Under Load Permanent</u> 5.61 0.54	PASS



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Test Date: October 6, 2020

Table (3): Impact Load and Post-Breakage Retention Test Results

(CSA A500-16, 5.5.3.1)

(CSAA500-10, 5.5.5.1)				
TEST REQUIREMENT	Load Location	RESULTS	RATING	
Combination Load Service +Wind Loads	Large Panel	Panel Width: 1067 mm (42")		
	Service Load:	Panel Height: 304 mm (12")		
Service Load: 3.75 kN (843lb)	Mid-span of top	Equivalent wind load applied on		
Wind Pressure : 2.5 kPa (50 psf) No maximum criteria provided for		panel: 3.75 kN (843 lb) Top Rail deflection (mm)	PASS	
deflection under load or for permanent	Wind Load:	•		
deflection after loading.	Center of infill aluminum panel	<u>Under Load Permanent</u> 20.58 2.96		
Impact Load and Post-Breakage				
Retention Testing				
Impact Location: Center of Panel	Panel 1	Center of lower aluminum panel,	PASS	
Impactor Weight: 45.4 kg (100 lb)	ranei i	No upper glass breakage	TASS	
Drop Height: 1220 mm (48")				
Performance Criteria				
 5.5.3.1 (b) i) the compromised panel or infill, including all parts and components, remains in place; ii) the impact does not create an opening in the panel or infill through which a 150 mm sphere is able to pass under an 18 N load; and 	Panel 2	Center of lower aluminum panel, No upper glass breakage	PASS	
iii) the compromised panel, infill, or system is able to withstand a load in the direction of the impact that is equivalent to 25% of the service wind load or 225 N, whichever is greater. This load may be applied in the form of a point load acting in the center of the panel or infill.	Panel 3	Center of lower aluminum panel, No upper glass breakage	PASS	

6. CONCLUSION

Based on the observations and obtained test results, the balcony railing system described in this report **DID MEET** the load and impact resistance requirements specified in OBC/NBCC (Article 4.1.5.14) and Article 5.5.3.1 of the CSA A500-16 *Standard "Building Guards"*.

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Report History

Revision No.	Change and Reason	Date	Approved by
	Original report issued	January 4, 2021	EA



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Appendix (A) - CSA A500-16 Requirements

5.3.2 Effect of service load and ultimate load as per Article 4.1.5.14. of the NBC

The guard assembly and components shall be tested for the effect of service load and ultimate load.

The difference between the deflection after the peak service load has been released and the deflection at "zero". Load shall be less than 5 mm and shall not increase with repeated application of the peak service load.

5.3.3 Effect of total ultimate load, including load combinations

The guard assembly configuration in which the applied loading produces the highest component stresses under the applied load, including all load-bearing components and connections, shall be tested for the effect of total ultimate load, including load combinations in accordance with Table 4.3.

5.3.4 Response to loads

After final installation, a minimum of one specimen per each type of guard assembly and its components, on which the applied load exerts the most adverse stress condition, shall be tested for response to loads in accordance with the NBC, Article 4.1.5.14. Appropriate load and resistance factors shall be used.

material factors of 0.8 for aluminum and 0.85 for glass i

5.3.5 Effect of total ultimate on main connections

After final installation on site, a minimum of two main connections of the guard assembly to the supporting structure shall be tested on site for the effect of the total ultimate load including load combinations in accordance with Table 4.3.5.3.6 Impact and post breakage retention testing

5.3.6 Impact Load

5.3.6.1

The infill panel in the guard assembly or the panel portion of the guard assembly shall be tested as part of the assembly for the impact load, unless sufficient documented proof is provided to verify that the system is capable of sustaining the impact test load.

5.3.6.2

Infills need not be tested under impact for guard assemblies that

- a) are walls acting as guards; or
- b) comprise
 - i) parapet that is
 - 1) adequately designed to support all the loads applied to the guard and transfers them to the supporting structure; and
 - 2) 500 mm or higher above the surface the guard is intended to protect; and
 - ii) a guard infill that is supported on at least two opposite sides.

5.5.1.9 Pass/Fail Criteria

If one or more of the below conditions is noted on the tested assembly prior to reaching the required peak "test load", the test shall be considered as having failed:

- a) full or partial pull out of anchors connecting the guard assembly and its components to the supporting structure;
- b) the assembly or any of its components exhibit excessive deformation, fracture, pull-out, or breakage of the supporting structure;
- c) deformation of the assembly or any of its components will increase without increasing the applied load;
- d) the assembly or any of its components exhibit evidence of yielding;
- e) any part or component of the assembly fails to remain in its design position; or
- f) the assembly or any of its components fail to remain connected.



Appendix (A) - CSA A500-16 Requirements

5.5.2 Impact and post-breakage retention testing

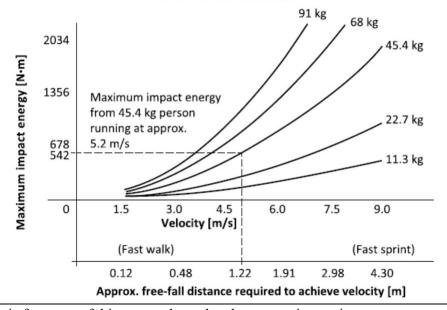
Impact testing shall be carried out as a minimum on the longest and the shortest infill spans for which compliance is sought.

For each guard configuration under test, impact testing shall be performed on three identical specimens according to the following criteria:

- a) for simple, in-line, assemblies, each specimen shall include two posts, one infill panel (or pickets), anchors, and associated components; or
- b) for assemblies having infill panels or pickets overhanging the supporting structure or end post, each specimen shall include two posts, with the infill panel or pickets configured to match the proposed overhanging design and including end supports and associated system components where applicable.

Figure A.1 Human engineering data for impact energies

(See Clause A.5.5.2.)



5.5.3 Criteria for successful impact and post-breakage retention testing

The impact and post-breakage retention test shall be deemed as passed if one of the following conditions are met:

- a) the panel or infill remains intact after impact; or
- b) the panel or infill integrity is compromised by the impact and all of the following conditions are met:
 - i) the compromised panel or infill, including all parts and components, remains in place;
 - ii) the impact does not create an opening in the panel or infill through which a 150 mm sphere is able to pass under an 18 N load; and
 - iii) the compromised panel, infill, or system is able to withstand a load in the direction of the impact that is equivalent to 25% of the service wind load or 225 N, whichever is greater. This load may be applied in the form of a point load acting in the center of the panel or infill.



